the implicit step is an efficient and rapid way of obtaining reasonably accurate solutions.

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Optimal Barrel-Shaped Shells Under Buckling Constraints

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I. Introduction

DOUBLY curved shells are common structural elements in various industries (aerospace, marine, civil engineering, etc.), and the buckling of such elements has received much attention in design. ¹⁻⁶ It is generally agreed that the papers by McElman and Stein^{1,2} sparked off detailed analysis of barreled shells. Their theory was limited to shallow shells and to buckling near the equator of a toroidal shell segment. Solutions were obtained, in a closed analytical form, for simply supported shells of both positive and negative curvature under internal or external pressure. A finite-element solution, as well as some experimental results presented in Ref. 7, did not completely agree with these first solutions.

The buckling and postbuckling behavior of axially compressed bowed-out and bowed-in shells was discussed in Ref.

8. Linear shell theory was used to find the buckling load, whereas the nonlinear theory was adopted to investigate the postbuckling state.

Qualitative ways of increasing the buckling resistance of barreled shells via meridional curvature change, stiffener eccentricity, and load eccentricity have been pointed out in Refs. 9-14. A substantial increase in the resistance of these shells to buckling has been observed, although these studies have not been based on optimal configurations.

The minimum weight of stiffened and barreled shells was examined in Ref. 15. In order to obtain the minimum weight configuration for a fixed axial load, the thickness of the skin and the thickness, depth, and spacing of solid rectangular stiffening rings and stringers were sought. Buckling was based on the linear membrane prebuckling state, and the gradient method with a penalty function was used.

The main aim of this paper is to establish, parametrically, the maximal compressive axial buckling load of a bowed-out shell. The wall thickness and the shell volume are kept constant. The buckling load increase is due to the change in the shell meridional curvature.

II. Formulation of Problem

Consider an elastic, clamped-clamped cylindrical shell of radius R_0 , length L_0 , constant thickness t, under axial compression F (see Fig. 1a). For a given volume of the cylindrical shell $V_{\rm cyl}$, we seek the meridional radius of curvature r of the toroidal shell (see Fig. 1b) which possesses the same volume, thickness, boundary conditions, and truncation radius R_0 , and also maximizes the buckling load.

III. Method of Solution

The volume of the toroidal shell (Fig. 1b) can be written as

$$V_{\text{tor}} = 4\pi (r^2 \sin \alpha - br\alpha)t \tag{1}$$

For a given value of the distance b, from the axis of revolution to the center of meridional curvature (see Fig. 1b), one can find the radius r from

$$V_{\text{tor}} = V_{\text{cvl}} \tag{2}$$

or

$$2(r^2 \sin\alpha - br\alpha) = R_0 L_0 \tag{3}$$

where R_0, L_0 are supposed to be given.

For a given value of the control parameter b we obtain, via Eq. (3), all the necessary geometrical parameters of the toroidal shell, which has the same volume as the original cylindrical shell. The computer code BOSOR 4^{16} is used here to find the bifurcation buckling load and the collapse load. The latter is found by tracing out the load-displacement curve beyond the limit point (applying displacement-controlled analysis). Varying b in the range from zero to infinity, one obtains the truncated sphere and cylinder, respectively, as limiting cases.

IV. Numerical Results

Calculations were performed for L_0/R_0 varying from 0.25 to 4.0 and R_0/t varying from 100 to 1000. Figure 2 shows the change of bifurcation load as a function of curvature. For $L_0/R_0=4$, the optimal load is obtained for $R_0/(R_0+b)=0.09$, regardless of the parameter R_0/t within the range of 100-1000. Analogous values for $L_0/R_0=2$ and $L_0/R_0=1$ are 0.17 and 0.28, respectively. Additional results for $L_0/R_0=0.5$ and $L_0/R_0=0.25$ are depicted in Fig. 3. It is seen that the optimal configurations do not depend on R_0/t for $0.5 \le L_0/R_0 \le 4.0$. However, for $L_0/R_0=0.25$ and $R_0/t < 300$, a change in meridional curvature no longer increases the buckling load. Axisymmetric collapse now be-

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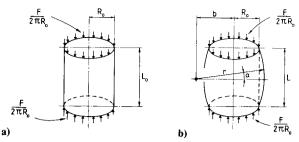


Fig. 1 Geometry of an a) cylindrical and b) barreled-out shell.

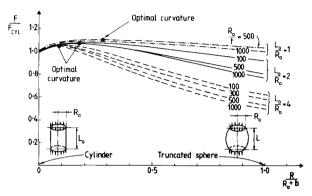


Fig. 2 Bifurcation load vs control variable $R_0/(R_0+b)$ for $L_0/R_0=1,\ 2,\ {\rm and}\ 4.$

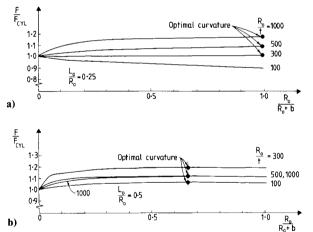


Fig. 3 Bifurcation load vs control variable $R_0/(R_0+b)$ for a) $L_0/R_0=0.25$ and b) $L_0/R_0=0.5$.

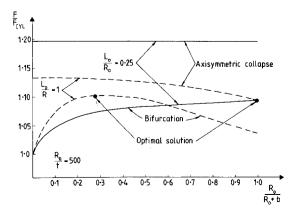


Fig. 4 Bifurcation and axisymmetric collapse loads vs amount of barreling out for $L_0/R_0=1$ and $L_0/R_0=0.25$.

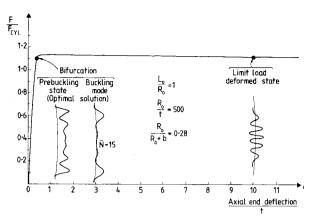


Fig. 5 Load-deflection behavior of an optimally bowed-out shell.

comes the controlling mode, and the collapse load decreases as the amount of barreling out increases (see Fig. 3a). Limit and bifurcation loads are displayed together in Fig. 4, for some of the parameters investigated heretofore. The load-displacement curve is shown for the optimal value of the control variable $R_0/(R_0+b)=0.28$ in Fig. 5. The bifurcation and limit loads are denoted on this curve. The prebuckling state and buckling mode are added as inserts for the bifurcation load; the deformed state is shown for the limit load.

V. Conclusions

As a result of this study, the following observations can be made:

- 1) The controlling mode of failure is bifurcation buckling for $0.5 \le L_0/R_0 \le 4$, and the optimal critical load does not depend on R_0/t . For shorter cylinders (e.g., $L_0/R_0 = 0.25$), the controlling mode is either bifurcation buckling $(R_0/t \le 300)$ or axisymmetric collapse $(R_0/t < 300)$.
- 2) There is no dramatic increase in the buckling load, although gains of up to 20% can be achieved via the curvature shaping.
- 3) Barreling out does not necessarily lead to an increase in buckling load, as is commonly assumed in the literature.
- 4) There is a wide plateau of curvature values where a small change in the curvature distribution does not cause a big change in the load-carrying capacity for $0.25 \le L_0 \div R_0 \le 1.0$.

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